

## A Suggested Analytical Solution For Laminated Closed Cylindrical Shells Using General Third Shell Theory (G.T.T.)

Muhsin J. Jweeg, Ph. D.,
Wedad I. Alazzawy, Ph. D.

## - Nomenclature

| a, b | Dimensions of shell. |
| :---: | :---: |
| $\mathrm{A}_{\mathrm{mn}}, \mathrm{B}_{\mathrm{mn}}, \mathrm{C}_{\mathrm{mn}}, \mathrm{D}_{\mathrm{mn}}$, | Arbitrary constants |
| $\mathrm{E}_{\mathrm{mn}}, \mathrm{F}_{\mathrm{mn}}, \mathrm{J}_{\mathrm{mn}}$ |  |
| B | constant |
| $\mathrm{C}_{\mathrm{ij}}$ | stiffness matrix elements |
| $\mathrm{E}_{1}, \mathrm{E}_{2}, \mathrm{E}_{3}$ | Elastic Modulus components (GPa) |
| $\mathrm{f}_{\mathrm{mn}}(\mathrm{t})$ | Generalized force (N) |
| $\mathrm{g}_{1}, \mathrm{~g}_{2}$ | Body forces (N) |
| $\mathrm{G}_{12}, \mathrm{G}_{13}, \mathrm{G}_{23}$ | Shear modulus components (GPa) |
| H | Thickness (mm) |
| K | Kinetic energy |
| L | Cylinder length (mm) |
| m, n | indices |
| $\begin{array}{ll} \left.\mathrm{N}_{\mathrm{i}}, M_{i}, 2,3,6\right) \end{array} \quad \mathrm{P}_{\mathrm{i}}, \quad \mathrm{~S}_{\mathrm{i}}$ | Resultant reactions $(\mathrm{N} / \mathrm{mm}),(\mathrm{N} . \mathrm{mm})$ |
| $\mathrm{m}_{1}, \mathrm{~m}_{2}, \mathrm{~m}_{3}, \mathrm{~m}_{4}$ | Body moments (N.mm) |
| $\mathrm{Q}_{\mathrm{i}}, \mathrm{K}_{\mathrm{i}}(\mathrm{i}=4,5)$ | Resultant reactions |
| $\mathrm{Q}_{\mathrm{ij}}$ | Elastic stiffness coefficients |
| q | Distributed transverse load ( $\mathrm{N} / \mathrm{mm}^{2}$ ) |
| R | Cylinder radius (mm) |
| $\mathrm{R}_{11}, \mathrm{R}_{22}$ | Principal radii of curvature of shell (mm) |
| U | Potential energy |
| $\begin{aligned} & \mathrm{u}, \mathrm{v}, \mathrm{w}, \varphi_{1}, \varphi_{2}, \psi_{1}, \\ & \psi_{2}, \psi_{3}, \theta_{1}, \theta_{2}, \theta_{3} \end{aligned}$ | Displacement components (mm) |
| z | Distance from neutral axis (mm) |
| $\varepsilon_{1,2,3,4,5,6}$ | Strain components in |

> principal directions
> Poison's ratios
> Density $\left(\mathrm{Ns}^{2} / \mathrm{mm}^{4}\right)$
> Frequency $(\mathrm{rad} / \mathrm{s})$
> Stress components (MPa)
> in principal direction


#### Abstract

: Transient solutions will be developed for laminated simply supported closed cylindrical shells subjected to a uniform dynamic pressure at the outer surface of the cylinder. These solutions are obtained by using General Third Shell Theory (G.T.T.). Rectangular pulse, triangular pulse, sinusoidal pulse and (ramp-constant) load-time varying functions are studied and the required equilibrium equations are developed. The central deformation and principle stresses are investigated for different cross-ply laminates.


## Keywords: Laminate, Cylindrical, Shells.

## 1.Introduction

With the increasing use of composite materials in many industries and especially in high performance aircraft industry, there is a need for assessing the response of laminated cylindrical shells to dynamic loading.

Analytical description of laminated composite shell is often based on classical laminate shell theory, which is an extension of the Love-Kirchhoff shell theory to composite shells. In Classical Shell Theory
(CST), the transverse strains are neglected under the assumption that straight lines normal to the middle surface are rigid. The neglect of transverse strains in composite laminates could lead to underestimation of deflections and overestimation of natural frequencies and critical buckling loads because of the very high transverse shear modulus compared to the in-plane Young's modulus [1].

High order shell theories are those in which the transverse strains are accounted. Y. Narita et al [2] developed a theoretical method for solving the free vibration angle-ply laminated cylindrical shells. The angle-ply laminated shell is macroscopically modeled as a thin shell of General anisotropy by using the classical lamination theory. The Functional derived from the flugge-type shell theory is minimized by following the Ritz procedure, and arbitrary combinations of boundary conditions at both ends are accommodated by introducing newly developed admissible functions.
Z.C.Xi et al [3] investigated the effects of shear non-linearity on free vibration of laminated composite shells of revolution using a semi-analytical method based on Reissner-Mindlin shell theory. The coupling between symmetric and anti-symmetric vibration modes of the shell is considered in the shear deformable shell element. Aleksandr Korjakin et al [4] used zig-zag model to investigate the free damped vibration of sandwich shells of revolution. As special cases the vibration analysis under consideration of damping of cylindrical, conical and spherical sandwich shells is performed. A specific sandwich shell finite element with 54 degrees of freedom is employed. Werner Hufenbach et al [5] developed an analytical solution for lightweight design using dynamically loaded fiber-reinforced composite shells. The analytic results were fully corroborated by accompanying FE calculations for special lay-ups. Humayun R. H. Kabir [6] investigated analytically the free vibration response of an arbitrarily laminated (crafted with advanced fiber reinforced composite materials)-thin and shallow cylindrical panels on rectangular planform with simply supported boundary conditions, using Kirchhoff-Love theory. J. J. Lee et al [7] used the finite element method based on Hellinger-Reissner principle with independent strain to analyze the vibration problem of cantilevered twisted plates, cylindrical and conical laminated shells. M. Amabili [8] investigated largeamplitude (geometrically non-linear) vibrations of circular cylindrical shells subjected to radial harmonic excitation in the spectral neighborhood of lowest resonance. Young-Shin Lee et al [9] investigated the free vibration analysis of a laminated composite cylindrical shell with an interior rectangular plate by analytical and experimental methods. The frequency equations of vibration of the shell including the plate are formulated by using the reacceptance method. S. C. Pradhan \& J. N. Reddy [10] presented an analytical solution of laminated
composite shells with embedded actuating layers. The magnetostrictive actuating layers are used to control natural vibration of laminated composite shell panels. The (FSDT) is used to represent the shell kinematics and equations of motion. Ghanim Shaker [11] presented a general content of the classical composite cylindrical shell theory and the first order shell theory for elasto-static and elasto-dynamic analysis of shells of circular cross section, incorporating for the first time the gathered effects of internal (and external) pressure, thermal gradient, axial end loading, and the conventional end condition, on the mechanical behavior of the structure. M. Darvizeh et al [12] presented a calculation of overall dynamic response of thin orthotropic cylindrical shells. Due to the obvious importance of the effects of transverse shear deformation and rotary inertia, these terms are included in the analysis. The exact method is modified to predict the dynamic behavior of an orthotropic circular cylindrical shell. In this work the developed analytical solution includes deriving the equation of motion using GTT for the first time to analyze displacement and stress components for forced vibration of laminated composite cylindrical shells.

## 2. Equations of motion:

In present study the high-order theory displacement field is:


$$
\begin{align*}
& \varepsilon_{1}=\frac{\partial \mathrm{u}}{\partial \mathrm{~L}}, \varepsilon_{2}=\frac{1}{\mathrm{R}} \times\left(\frac{\partial}{\partial \theta}\right)+\frac{\mathrm{w}}{\mathrm{R}}, \varepsilon_{3}=\frac{\partial \mathrm{v}}{\partial \mathrm{z}}, \varepsilon_{6}=\frac{1}{\mathrm{R}} \times\left(\frac{\partial \mathrm{\partial}}{\partial \theta}\right)+\frac{\partial \mathrm{v}}{\partial \mathrm{x}} \\
& \varepsilon_{5}=\frac{\partial \mathrm{u}}{\partial z}+\frac{\partial \mathrm{w}}{\partial \mathrm{x}}  \tag{2}\\
& \varepsilon_{4}=\frac{\partial}{\partial z}-\frac{\mathrm{v}}{\mathrm{R}}+\frac{1}{\mathrm{R}} \times\left(\frac{\partial \mathrm{v}}{\partial \theta}\right)
\end{align*}
$$

and hence transverse strain also vanishes, so:

$$
\begin{aligned}
& \varepsilon_{5}(\mathrm{x}, \theta, \pm \mathrm{h} / 2, \mathrm{t})=\frac{\partial \mathrm{u}}{\partial \mathrm{z}}+\frac{\partial \mathrm{w}}{\partial \mathrm{x}}=0 \\
& \varepsilon_{4}(\mathrm{x}, \theta, \pm \mathrm{h} / 2, \mathrm{t})=\frac{\partial \mathrm{v}}{\partial \mathrm{z}}-\frac{\mathrm{v}}{\mathrm{R}}+\frac{1}{\mathrm{R}} \times\left(\frac{\partial \mathrm{w}}{\partial \theta}\right)=0
\end{aligned}
$$

The resulting strain-displacement relations are:

Assuming that transverse shear stress vanishing at top and bottom of the laminated composite layers,

$$
\begin{aligned}
& \varepsilon_{1}=\frac{\partial \mathrm{u}_{0}}{\partial \mathrm{x}}+\mathrm{z} \times \frac{\partial \phi_{1}}{\partial \mathrm{x}}-\mathrm{z}^{2} \times \frac{\partial^{2} \psi_{3}}{\partial \mathrm{x}^{2}}-\mathrm{z}^{3} \times\left(\frac{4}{3 \times \mathrm{h}^{2}}\right) \times\left[\frac{\partial \phi_{1}}{\partial \mathrm{x}}+\frac{\partial^{2} \mathrm{w}_{0}}{\partial \mathrm{x}^{2}}+\left(\frac{\mathrm{h}^{2}}{4}\right) \times \frac{\partial^{2} \theta_{3}}{\partial \mathrm{x}^{2}}\right] \\
& \varepsilon_{2}=\left(\frac{1}{\mathrm{R}}\right) \times\left\{\begin{array}{l}
\left(\frac{\partial \mathrm{v}_{0}}{\partial \theta}+\mathrm{w}_{0}\right)+\mathrm{z} \times\left(\frac{\partial \phi_{2}}{\partial \theta}+\psi_{3}\right)+\mathrm{z}^{2} \times\left(\left(\frac{-1}{2 \times \mathrm{R}}\right) \times \frac{\partial^{2} \psi_{3}}{\partial \theta^{2}}+\theta_{3}\right) \\
-\mathrm{z}^{3} \times\left(\frac{4}{3 \times \mathrm{h}^{2}}\right) \times\left[\frac{\partial \phi_{2}}{\partial \theta}-\left(\frac{1}{\mathrm{R}}\right) \times \frac{\partial \mathrm{v}_{0}}{\partial \theta}+\left(\frac{1}{\mathrm{R}}\right) \times \frac{\partial^{2} \mathrm{w}_{0}}{\partial \theta^{2}}+\left(\frac{\mathrm{h}^{2}}{4 \mathrm{R}}\right) \times \frac{\partial^{2} \theta_{3}}{\partial \theta^{2}}\right] \\
\varepsilon_{3}=\psi_{3}+2 \times \mathrm{z} \times \theta_{3} \\
\varepsilon_{6}=\frac{\partial \mathrm{v}_{0}}{\partial \mathrm{x}}+\left(\frac{1}{\mathrm{R}}\right) \times \frac{\partial \mathrm{u}_{0}}{\partial \theta}+\mathrm{z} \times\left(\left(\frac{1}{R}\right) \times \frac{\partial \phi_{1}}{\partial \theta}+\frac{\partial \phi_{2}}{\partial \mathrm{x}}\right)-\left(\frac{\mathrm{z}^{2}}{\mathrm{R}}\right) \times \frac{\partial^{2} \psi_{3}}{\partial \mathrm{x} \partial \theta} \\
-\mathrm{z}^{3} \times\left(\frac{4}{3 h^{2}}\right) \times\left[\left(\frac{\partial \phi_{2}}{\partial \mathrm{x}}+\left(\frac{1}{\mathrm{R}}\right) \times \frac{\partial \phi_{1}}{\partial \theta}\right)-\left(\frac{1}{\mathrm{R}}\right) \times \frac{\partial \mathrm{v}_{0}}{\partial \mathrm{x}}+\left(\frac{2}{\mathrm{R}}\right) \times \frac{\partial^{2} \mathrm{w}_{0}}{\partial \mathrm{x} \partial \theta}+\left(\frac{\mathrm{h}^{2}}{2 \mathrm{R}}\right) \times \frac{\partial^{2} \theta_{3}}{\partial \mathrm{x} \partial \theta}\right] \\
\varepsilon_{4}=\left(\phi_{2}-\frac{\mathrm{v}_{0}}{\mathrm{R}}+\left(\frac{1}{\mathrm{R}}\right) \times \frac{\partial \mathrm{v}_{0}}{\partial \theta}\right)-{z^{2}}_{2} \times\left(\frac{4}{\mathrm{~h}^{2}}\right) \times\left(\phi_{2}-\frac{\mathrm{v}_{0}}{\mathrm{R}}+\left(\frac{1}{\mathrm{R}}\right) \times \frac{\partial \mathrm{w}_{0}}{\partial \theta}\right) \\
\varepsilon_{5}=\phi_{1}+\frac{\partial \mathrm{v}_{0}}{\partial \mathrm{x}}-\mathrm{z}^{2} \times\left(\frac{4}{\mathrm{~h}^{2}}\right) \times\left(\phi_{1}+\frac{\partial \mathrm{v}_{0}}{\partial \mathrm{x}}\right)
\end{array}\right.
\end{aligned}
$$

According to Hamilton's' Principles:

| $\int_{\mathrm{t} 2}^{\mathrm{t} 1}(\delta \mathrm{U}-\delta K) \mathrm{dt}=0$ | $\mathbf{4}$ |
| :--- | :--- |

where:
$U=\iint_{\mathrm{Az}}\left(\sigma_{1} \delta \tilde{q}_{1}+\sigma_{2} \delta \varepsilon_{2}+\sigma_{3} \delta \xi_{\xi}+\sigma_{6} \delta \varepsilon_{\xi}+\sigma_{5} \delta \xi_{\xi}+\sigma_{4} \delta \varepsilon_{4}\right) * \mathrm{Rd}$ \&lzd:
$\delta \mathrm{K}=-\int_{\mathrm{t}} \iiint_{\mathrm{V}} \mathrm{R} \rho(\ddot{\mathrm{u}} \delta \mathrm{u}+\ddot{\mathrm{v}} \delta \mathrm{v}+\ddot{\mathrm{w}} \delta \mathrm{w}) \mathrm{dzdxd} \theta \mathrm{dt}$
From above 7equations of motion the following equations are obtained:

$$
\frac{\partial \mathrm{N}_{1}}{\partial \mathrm{x}}+\left(\frac{1}{\mathrm{R}}\right) \frac{\partial \mathrm{N}_{6}}{\partial \theta}+\mathrm{g}_{1}=\mathrm{I}_{1} \ddot{\mathrm{u}}+\mathrm{I}_{2} \ddot{\phi}_{2}-\left(\frac{\mathrm{I}_{3}}{2}\right) \frac{\partial \ddot{\psi_{3}}}{\partial \mathrm{x}}-\left(\frac{4 \mathrm{I}_{4}}{3 \mathrm{~h}^{2}}\right)\left(\ddot{\phi_{1}}+\frac{\partial \ddot{\mathrm{w}}}{\partial \mathrm{x}}\right)-\left(\frac{\mathrm{I}_{4}}{3}\right) \frac{\partial \ddot{\theta}_{3}}{\partial \mathrm{x}}
$$

$$
\begin{aligned}
& \frac{\partial \mathrm{N}_{2}}{\partial \theta}+\mathrm{R} \frac{\partial \mathrm{~N}_{6}}{\partial \mathrm{x}}+\mathrm{Q}_{4}-\left(\frac{4}{\mathrm{~h}^{2}}\right) \mathrm{K}_{4}+\left(\frac{4}{3 \mathrm{~h}^{2}}\right)\left(\left(\frac{1}{\mathrm{R}}\right) \frac{\partial \mathrm{S}_{2}}{\partial \theta}+\frac{\partial \mathrm{S}_{6}}{\partial \mathrm{x}}\right)+\mathrm{g}_{2}=\left(\mathrm{RI}_{2}-\left(\frac{4 \mathrm{RI}_{4}}{3 \mathrm{~h}^{2}}\right)-\left(\frac{16 \mathrm{I}_{7}}{9 \mathrm{~h}^{4} \mathrm{R}}\right)+\left(\frac{4 \mathrm{I}_{5}}{3 h^{2}}\right)\right) \ddot{\phi}_{2}+ \\
& \left(\mathrm{RI}_{1}+\left(\frac{8 \mathrm{I}_{4}}{3 \mathrm{~h}^{2}}\right)+\left(\frac{16 \mathrm{I}_{7}}{9 \mathrm{~h}^{4} \mathrm{R}}\right)\right) \ddot{\mathrm{v}}+-\left(\left(\frac{\mathrm{I}_{3}}{2}\right)+\left(\frac{4 \mathrm{I}_{6}}{6 \mathrm{Rh}^{2}}\right)\right) \frac{\partial \ddot{\psi_{3}}}{\partial \theta}+\left(\left(\frac{4 \mathrm{I}_{4}}{3 \mathrm{~h}^{2}}\right)-\left(\frac{16 \mathrm{I}_{7}}{9 \mathrm{~h}^{4} \mathrm{R}}\right)\right) \frac{\partial \ddot{\mathrm{w}}}{\partial \theta}-\left(\frac{\mathrm{I}_{4}}{3}+\left(\frac{4 \mathrm{I}_{7}}{9 \mathrm{~h}^{2} \mathrm{R}}\right)\right) \frac{\partial \ddot{\theta}_{3}}{\partial \mathrm{x}}
\end{aligned}
$$

$$
\begin{aligned}
& \left(\frac{4}{3 h^{2}}\right)\left(\frac{1}{\mathrm{R}} \frac{\partial^{2} \mathrm{~S}_{2}}{\partial \theta^{2}}+\mathrm{R} \frac{\partial^{2} \mathrm{~S}_{1}}{\partial \mathrm{x}^{2}}+2 \frac{\partial^{2} \mathrm{~S}_{6}}{\partial \mathrm{x} \partial \theta}\right)-\mathrm{N}_{2}-\left(\frac{4}{\mathrm{~h}^{2}}\right)\left(\frac{\partial \mathrm{K}_{4}}{\partial \theta}+\mathrm{R} \frac{\partial \mathrm{~K}_{5}}{\partial \mathrm{x}}\right)+\left(\frac{\partial \mathrm{Q}_{4}}{\partial \theta}+\mathrm{R} \frac{\partial \mathrm{Q}_{5}}{\partial \mathrm{x}}\right)+\mathrm{q}=\mathrm{RI}_{1} \ddot{\mathrm{w}}+ \\
& \left(-\left(\frac{16 \mathrm{I}_{7}}{9 \mathrm{~h}^{4} \mathrm{R}}\right)+\left(\frac{4 \mathrm{I}_{5}}{3 \mathrm{~h}^{2}}\right)\right) \frac{\partial \ddot{\phi}_{2}}{\partial \theta}+\mathrm{RI}_{2} \ddot{\psi_{3}}-\left(\frac{4 \mathrm{I}_{6}}{6 \mathrm{Rh}^{2}}\right) \frac{\partial^{2} \ddot{\psi_{3}}}{\partial \theta^{2}}+\left(\frac{4 \mathrm{I}_{4}}{3 \mathrm{~h}^{2}}\right)-\left(\frac{16 \mathrm{I}_{7}}{9 \mathrm{~h}^{4}}\right)\left(\left(\frac{1}{\mathrm{R}}\right) \frac{\partial^{2} \ddot{\mathrm{w}}}{\partial \theta^{2}}+\mathrm{R} \frac{\partial^{2} \ddot{\mathrm{w}}}{\partial \mathrm{x}^{2}}\right)+\mathrm{RI}_{3} \ddot{\theta_{3}} \\
& -\left(\frac{4 \mathrm{I}_{7}}{9 \mathrm{~h}^{2} \mathrm{R}}\right)\left(\mathrm{R} \frac{\partial^{2} \ddot{\theta}_{3}}{\partial \mathrm{x}^{2}}+\frac{1}{\mathrm{R}} \frac{\partial^{2} \ddot{\theta}_{3}}{\partial \mathrm{x}^{2}}\right)+\mathrm{R}\left(-\left(\frac{16 \mathrm{I}_{7}}{9 \mathrm{~h}^{4} \mathrm{R}}\right)+\left(\frac{4 \mathrm{I}_{5}}{3 h^{2}}\right)\right) \frac{\partial \ddot{\phi}_{1}}{\partial \mathrm{x}}+\left(\left(\frac{4 \mathrm{I}_{4}}{3 \mathrm{~h}^{2}}\right)+\left(\frac{16 \mathrm{I}_{7}}{9 \mathrm{~h}^{4} \mathrm{R}}\right)\right) \frac{\partial \ddot{\mathrm{v}}}{\partial \theta}+\left(\frac{4 \mathrm{I}_{4}}{3 \mathrm{~h}^{2}}\right) \frac{\partial \ddot{\mathrm{u}}}{\partial \mathrm{x}}
\end{aligned}
$$

$$
\begin{aligned}
& \mathrm{R} \frac{\partial \mathrm{M}_{1}}{\partial \mathrm{x}}-\left(\frac{4 \mathrm{R}}{3 \mathrm{~h}^{2}}\right) \frac{\partial \mathrm{S}_{1}}{\partial \mathrm{x}}+\frac{\partial \mathrm{M}_{6}}{\partial \theta}-\left(\frac{4 \mathrm{R}}{3 \mathrm{~h}^{2}}\right) \frac{\partial \mathrm{S}_{6}}{\partial \theta}-\mathrm{RQ}_{5}+\left(\frac{4 \mathrm{R}}{\mathrm{~h}^{2}}\right) \mathrm{K}_{5}=\left(\mathrm{RI}_{2}-\frac{4 \mathrm{RI}_{4}}{3 \mathrm{~h}^{2}}\right) \ddot{\mathrm{u}}+\left(\mathrm{RI}_{3}-\frac{8 \mathrm{RI}_{5}}{3 \mathrm{~h}^{2}}+\frac{16 \mathrm{RI}_{7}}{9 \mathrm{~h}^{4}}\right) \ddot{\phi_{1}}+ \\
& \left(\frac{4 \mathrm{RI}_{6}}{6 \mathrm{~h}^{2}}-\frac{\mathrm{RI}_{4}}{2}\right) \frac{\partial \psi_{3}}{\partial \mathrm{x}}+\left(-\frac{8 \mathrm{RI}_{5}}{3 \mathrm{~h}^{2}}+\frac{16 \mathrm{RI}_{7}}{9 \mathrm{~h}^{4}}\right) \frac{\partial \mathrm{w}}{\partial \mathrm{x}}+\left(-\frac{\mathrm{RI}_{5}}{3}+\frac{4 \mathrm{RI}_{7}}{9 \mathrm{~h}^{2}}\right) \frac{\partial \ddot{\theta}_{3}}{\partial \mathrm{x}}
\end{aligned}
$$

$$
\begin{align*}
& \frac{\partial M_{2}}{\partial \theta}-\left(\frac{4 R}{3 h^{2}}\right) \frac{\partial S_{2}}{\partial \theta}+R \frac{\partial M_{6}}{\partial x}-\left(\frac{4 R}{3 h^{2}}\right) \frac{\partial S_{6}}{\partial x}-R Q_{4}+\left(\frac{4 R}{h^{2}}\right) K_{4}+m_{2}=\left(R I_{2}-\frac{16 R I_{7}}{9 h^{4}}\right) \ddot{v}+\left(R I_{3}-\frac{8 R I_{5}}{3 h^{2}}+\frac{16 R I_{7}}{9 h^{4}}\right) \ddot{\phi_{2}}+ \\
& \left(\frac{4 I_{6}}{6 h^{2}}-\frac{I_{4}}{2}\right) \frac{\partial \ddot{\psi_{3}}}{\partial \theta}+\left(-\frac{4 I_{5}}{3 h^{2}}+\frac{16 I_{7}}{9 h^{4}}\right) \frac{\partial \ddot{w}}{\partial \theta}+\left(-\frac{I_{5}}{3}+\frac{4 I_{7}}{9 h^{2}}\right) \frac{\partial \ddot{\theta}_{3}}{\partial \theta} \tag{9}
\end{align*}
$$

$$
\begin{aligned}
& \left(\frac{R}{2}\right) \frac{\partial^{2} P_{1}}{\partial x^{2}}+\left(\frac{1}{2 R}\right) \frac{\partial^{2} P_{2}}{\partial \theta^{2}}+\frac{\partial^{2} P_{6}}{\partial x \partial \theta}-R N_{3}-M_{2}=\left(\frac{R I_{3}}{2}\right) \frac{\partial \ddot{u}}{\partial x}+\left(\frac{R I_{4}}{2}-\frac{4 R I_{6}}{6 h^{2}}\right) \frac{\partial \ddot{\phi}_{1}}{\partial x}+\left(\frac{I_{3}}{2}+\frac{4 I_{6}}{6 R h^{2}}\right) \frac{\partial \ddot{v}}{\partial \theta} \\
& -\left(\frac{R I_{5}}{4}\right) \frac{\partial^{2} \psi_{3}}{\partial x^{2}}-\left(\frac{I_{5}}{4 R}\right) \frac{\partial^{2} \ddot{\psi_{3}}}{\partial \theta^{2}}+R I_{3} \psi_{3}+\left(-\frac{2 R I_{6}}{3 h^{2}}\right) \frac{\partial^{2} \ddot{w}}{\partial x^{2}}+\left(-\frac{4 I_{6}}{6 R h^{2}}\right) \frac{\partial^{2} \ddot{w}}{\partial \theta^{2}}+R I_{2} \ddot{w}+\left(-\frac{R I_{6}}{6}\right) \frac{\partial^{2} \ddot{\theta_{3}}}{\partial x^{2}}-\left(\frac{I_{6}}{6 R}\right) \frac{\partial^{2} \ddot{\theta}_{3}}{\partial \theta^{2}} \\
& +R I_{4} \ddot{\theta}_{3}+\left(\frac{I_{4}}{2}-\frac{4 I_{6}}{6 h^{2}}\right) \frac{\partial \ddot{\phi}_{2}}{\partial \theta}
\end{aligned}
$$

$\left(\frac{\mathrm{R}}{3}\right) \frac{\partial^{2} \mathrm{~S}_{1}}{\partial \mathrm{x}^{2}}+\left(\frac{1}{3 \mathrm{R}}\right) \frac{\partial^{2} \mathrm{~S}_{2}}{\partial \theta^{2}}+\left(\frac{2}{3}\right) \frac{\partial^{2} \mathrm{~S}_{6}}{\partial \mathrm{x} \partial \theta}-2 \mathrm{RM}_{3}-\mathrm{P}_{2}+\mathrm{m}_{4}=\left(\frac{\mathrm{RI}_{4}}{3}\right) \frac{\partial \ddot{\mathrm{u}}}{\partial \mathrm{x}}+\left(\frac{\mathrm{RI}_{5}}{3}-\frac{4 \mathrm{RI}_{7}}{9 h^{2}}\right) \frac{\partial \ddot{\phi_{1}}}{\partial \mathrm{x}}+\left(\frac{\mathrm{I}_{4}}{3}+\frac{4 \mathrm{I}_{7}}{9 \mathrm{Rh}^{2}}\right) \frac{\partial \ddot{\mathrm{v}}}{\partial \theta}$ $-\left(\frac{\mathrm{RI}_{6}}{6}\right) \frac{\partial^{2} \ddot{\psi}_{3}}{\partial \mathrm{x}^{2}}-\left(\frac{\mathrm{I}_{6}}{6 \mathrm{R}}\right) \frac{\partial^{2} \ddot{\psi}_{3}}{\partial \theta^{2}}+\mathrm{RI}_{4} \ddot{\psi}_{3}+\left(-\frac{4 \mathrm{RI}_{7}}{9 h^{2}}\right) \frac{\partial^{2} \ddot{\mathrm{w}}}{\partial \mathrm{x}^{2}}+\left(-\frac{4 \mathrm{I}_{7}}{9 \mathrm{Rh}^{2}}\right) \frac{\partial^{2} \ddot{\mathrm{w}}}{\partial \theta^{2}}+\mathrm{RI}_{3} \ddot{\mathrm{w}}+\left(-\frac{\mathrm{RI}_{7}}{9}\right) \frac{\partial^{2} \ddot{\theta}_{3}}{\partial \mathrm{x}^{2}}-\left(\frac{\mathrm{I}_{7}}{9 \mathrm{R}}\right) \frac{\partial^{2} \ddot{\theta}_{3}}{\partial \theta^{2}}$ $+\mathrm{RI}_{5} \ddot{\theta_{3}}+\left(\frac{\mathrm{I}_{5}}{3}-\frac{4 \mathrm{I}_{7}}{9 \mathrm{~h}^{2}}\right) \frac{\partial \ddot{\phi_{2}}}{\partial \theta}$


Substituting eqs.(12 and 13) in eq.(4) and then substituting the resultant forces and moments in equations of motion, the equations of motion are then solved by using Navier 's solution [2], which is presented as follows:
$u_{0}(x, \theta, z, t)=\sum_{m=1}^{\infty} \sum_{n=1}^{\infty} A_{m n} \cos \alpha x \sin \beta \theta^{*} T_{m n}(t)$
$v_{0}(x, \theta, z, t)=\sum_{m=1}^{\infty} \sum_{n=1}^{\infty} B_{m n} \sin \alpha x \cos \beta \theta^{*} T_{m n}(t)$
$w_{0}(x, \theta, z, t)=\sum_{m=1}^{\infty} \sum_{n=1}^{\infty} C_{m n} \sin \alpha x \sin \beta \theta^{*} T_{m n}(t)$
$\phi_{1}(x, \theta, z, t)=\sum_{m=1}^{\infty} \sum_{n=1}^{\infty} D_{m n} \cos \alpha x \sin \beta \theta^{*} T_{m n}(t)$
$\phi_{2}(x, \theta, z, t)=\sum_{m=1}^{\infty} \sum_{n=1}^{\infty} E_{m n} \sin \alpha x \cos \beta \theta^{*} T_{m n}(t)$
$\psi_{3}(x, \theta, z, t)=\sum_{m=1}^{\infty} \sum_{n=1}^{\infty} F_{m n} \sin \alpha x \sin \beta \theta^{*} T_{m n}(t)$
$\theta_{3}(x, \theta, z, t)=\sum_{m=1}^{\infty} \sum_{n=1}^{\infty} J_{m n} \sin \alpha x \sin \beta \theta^{*} T_{m n}(t)$

$$
\begin{aligned}
& w_{0}(x, \theta, z, t)=\sum_{m=1}^{\infty} \sum_{n=1}^{\infty} C_{m n} \sin \alpha x \sin \beta \theta^{*} T_{m n}(t) \\
& \phi_{1}(x, \theta, z, t)=\sum_{m=1}^{\infty} \sum_{n=1}^{\infty} D_{m n} \cos \alpha x \sin \beta \theta^{*} T_{m n}(t) \\
& \phi_{2}(x, \theta, z, t)=\sum_{m=1}^{\infty} \sum_{n=1}^{\infty} E_{m n} \sin \alpha x \cos \beta \theta^{*} T_{m n}(t) \\
& \psi_{3}(x, \theta, z, t)=\sum_{m=1}^{\infty} \sum_{n=1}^{\infty} F_{m n} \sin \alpha x \sin \beta \theta^{*} T_{m n}(t) \\
& \theta_{3}(x, \theta, z, t)=\sum_{m=1}^{\infty} \sum_{n=1}^{\infty} J_{m n} \sin \alpha x \sin \beta \theta^{*} T_{m n}(t)
\end{aligned}
$$

where:
$\alpha=\left(\frac{m \pi}{L}\right), \beta=n$
Amn, Bmn, Cmn, Dmn, Emn, Fmn, Jmn are arbitrary constants.

The stiffness and mass matrices will be obtained, then natural frequencies and their modes
are also computed by solving eignvalue problems shown below:

| $[\mathrm{C}]-\omega^{2}[\mathrm{M}]\{\mathrm{A}\}=0$ | $\mathbf{1 5}$ |
| :--- | :--- |

The orthogonality condition of principal modes can be established with the result as shown below:

|  | $\mathrm{d} \theta \mathrm{dx}=0$ | 16 |
| :---: | :---: | :---: |

The general distributed loads are expanded in a series of principal modes as follows:


$$
q=\sum f_{m n}(t) \times\left[\begin{array}{l}
\left(I_{1}+\frac{1 \boldsymbol{\sigma}_{7}}{9 h^{2}}\left(\alpha^{2}+\beta^{2}\right)\right) C_{m n}+\left(I_{2}+\frac{4 I_{6}}{6 h^{2}}\left(\alpha^{2}+\beta^{2}\right)\right) J_{m n} \\
\left.-\left(\frac{4 \alpha}{3 h^{2}}\right)\right)_{4} A_{m n}-\left(\frac{4 \beta}{3 h^{2}}\right)\left(I_{4}+\frac{I_{5}}{R_{2}}\right) B_{m n}-\left(\frac{4 \alpha}{3 h^{2}}\right)\left(I_{5}-\frac{4 I_{7}}{3 h^{2}}\right) D_{m n} \\
-\left(\frac{4 \beta}{3 h^{2}}\right)\left(I_{5}-\frac{4 I_{7}}{3 h^{2}}\right) E_{n n}+\left(I_{3}+\frac{4 I_{7}}{9 h^{2}}\left(\alpha^{2}+\beta^{2}\right)\right) F_{m n}
\end{array}\right]
$$



$$
\mathrm{m}_{2}=\sum \mathrm{f}_{\mathrm{mn}}(\mathrm{t}) \times\left[\begin{array}{l}
\left(\mathrm{I}_{2}+\frac{\mathrm{I}_{3}}{\mathrm{R}}-\frac{4}{3 \mathrm{~h}^{2}}\left(\mathrm{I}_{4}+\frac{\mathrm{I}_{5}}{\mathrm{R}}\right)\right) \mathrm{B}_{\mathrm{mn}}+\left(\mathrm{I}_{3}-\frac{8 \mathrm{I}_{5}}{3 \mathrm{~h}^{2}}+\frac{16 \mathrm{I}_{7}}{9 \mathrm{~h}^{4}}\right) \mathrm{D}_{\mathrm{mn}} \\
+\left(-\frac{\mathrm{I}_{4}}{2}+\frac{4 \mathrm{I}_{6}}{6 \mathrm{~h}^{2}}\right) \beta \mathrm{J}_{\mathrm{mn}}+\left(-\frac{4 \mathrm{I}_{5}}{3 \mathrm{~h}^{2}}+\frac{16 \mathrm{I}_{7}}{9 \mathrm{~h}^{4}}\right) \beta \mathrm{C}_{\mathrm{mn}}+\left(-\frac{\mathrm{I}_{5}}{3}+\frac{4 \mathrm{I}_{7}}{9 \mathrm{~h}^{2}}\right) \beta \mathrm{F}_{\mathrm{mn}}
\end{array}\right.
$$

$$
\mathrm{m}_{3}=\sum \mathrm{f}_{\mathrm{mn}}(\mathrm{t}) \times\left[\begin{array}{l}
\frac{4 \mathrm{I}_{6}}{6 \mathrm{~h}^{2}}\left(\alpha^{2}+\beta^{2}\right) \mathrm{C}_{\mathrm{mn}}+\frac{\mathrm{I}_{5}}{4}\left(\alpha^{2}+\beta^{2}\right) \mathrm{J}_{\mathrm{mn}}+\frac{\mathrm{I}_{6}}{6}\left(\alpha^{2}+\beta^{2}\right) \mathrm{F}_{\mathrm{mn}}-\left(\frac{\alpha}{2}\right) \mathrm{I}_{3} \mathrm{~A}_{\mathrm{mn}} \\
-\left(\frac{\beta}{2}\right)\left(\mathrm{I}_{3}+\frac{\mathrm{I}_{4}}{\mathrm{R}}\right) \mathrm{B}_{\mathrm{mn}}-\left(\frac{\alpha}{2}\right)\left(\mathrm{I}_{4}-\frac{4 \mathrm{I}_{6}}{3 \mathrm{~h}^{2}}\right) \mathrm{D}_{\mathrm{mn}}-\left(\frac{\beta}{2}\right)\left(\mathrm{I}_{4}-\frac{4 \mathrm{I}_{6}}{3 \mathrm{~h}^{2}}\right) \mathrm{E}_{\mathrm{mn}}
\end{array}\right.
$$

$$
\mathrm{m}_{4}=\sum \mathrm{f}_{\mathrm{mn}}(\mathrm{t}) \times\left[\begin{array}{l}
\left(\frac{4 \mathrm{I}_{7}}{9 \mathrm{~h}^{2}}\left(\alpha^{2}+\beta^{2}\right)+\mathrm{I}_{3}\right) \mathrm{C}_{\mathrm{mn}}+\frac{\mathrm{I}_{6}}{6}\left(\alpha^{2}+\beta^{2}\right) \mathrm{J}_{\mathrm{mn}}+\left(\frac{4 \mathrm{I}_{7}}{9 \mathrm{~h}^{2}}\left(\alpha^{2}+\beta^{2}\right)+\mathrm{I}_{5}\right) \mathrm{F}_{\mathrm{mn}} \\
-\left(\frac{1}{3}\right)\left(\mathrm{I}_{4}+\frac{\mathrm{I}_{5}}{\mathrm{R}}\right) \beta \mathrm{B}_{\mathrm{mn}}-\left(\frac{\alpha}{3}\right)\left(\mathrm{I}_{5}-\frac{4 \mathrm{I}_{7}}{3 \mathrm{~h}^{2}}\right) \mathrm{D}_{\mathrm{mn}}-\left(\frac{\beta}{3}\right)\left(\mathrm{I}_{5}-\frac{4 \mathrm{I}_{7}}{3 \mathrm{~h}^{2}}\right) \mathrm{E}_{\mathrm{mn}}-\left(\frac{\mathrm{I}_{4}}{3}\right) \alpha \mathrm{A}_{\mathrm{mn}}
\end{array}\right]
$$

The generalized forces $f_{m n}(t)$ are determined by making use of orthogonality condition. Multiplying eq. ((17)-a) by Amn, eq.( (17)-b) by Bmn, eq.( (17)-c) by Cmn, eq.( (17)-d) by Dmn, eq.( (17)-e) by Emn, eq.( (17)-f) by Jmn and eq.( (17)-g) by Fmn, and adding the results, integrating over the plane area, and taking into account eq. (16) leads to the following result:


Substituting eq.(14) into equations of motion, taking into account eq.(15), gives:


For any ( $\mathrm{m}, \mathrm{n}$ ).The solution to above equation is given by:

$$
\begin{equation*}
\mathrm{T}_{\mathrm{mn}}(\mathrm{t})=\frac{1}{\omega_{\mathrm{mn}}} \int_{0}^{\mathrm{t}} \mathrm{f}_{\mathrm{mn}}(\tau) \sin \omega_{\mathrm{mn}}(\mathrm{t}-\tau) \mathrm{d} \tau \tag{20}
\end{equation*}
$$

For sinusoidal spatial distribution of load, $q(x, \theta, t)=q 0 \sin \alpha x \sin \beta \theta F(t), \quad(m=n=1)$, the formal solution to the unknown functions may be expressed as:

| $\left(\begin{array}{l} \mathrm{u} \\ \mathrm{v} \\ \mathrm{w} \\ \phi_{1} \\ \phi_{2} \\ \psi_{3} \\ \theta_{3} \end{array}\right)$ | $=\sum_{\mathrm{k}=1}^{7} \frac{\mathrm{q}_{0}}{\mathrm{~N}_{\mathrm{mn}}(\mathrm{k}) \omega_{\mathrm{mn}}(\mathrm{k})}$ | $\left(\begin{array}{l} \mathrm{A}_{\mathrm{mn}}(\mathrm{k}) \mathrm{f}_{1}(\mathrm{x}, \theta) \\ \mathrm{B}_{\mathrm{mn}}(\mathrm{k}) \mathrm{f}_{2}(\mathrm{x}, \theta) \\ 1 \times \mathrm{f}_{3}(\mathrm{x}, \theta) \\ \mathrm{D}_{\mathrm{mn}}(\mathrm{k}) \mathrm{f}_{4}(\mathrm{x}, \theta) \\ \mathrm{E}_{\mathrm{mn}}(\mathrm{k}) \mathrm{f}_{5}(\mathrm{x}, \theta) \\ \mathrm{J}_{\mathrm{mn}}(\mathrm{k}) \mathrm{f}_{6}(\mathrm{x}, \theta) \\ \mathrm{F}_{\mathrm{mn}}(\mathrm{k}) \mathrm{f}_{7}(\mathrm{x}, \theta) \end{array}\right) \int_{0}^{\mathrm{t}} \mathrm{~F}(\tau) \sin \omega_{\mathrm{mn}}(\mathrm{k})(\mathrm{t}-\tau) \mathrm{d} \tau$ | 21 |
| :---: | :---: | :---: | :---: |

It is noted that the solution in eq.(21) is normalized with respect to $\mathrm{Cmn}(\mathrm{k})$, the coefficients in expansion of $w$.
where:


## 3. Numerical results

Two types of cross-ply and isotropic closed cylindrical shells are analyzed and their transient responses are evaluated numerically. Also a comparative study with a shallow shell of [1] is obtained analytically and numerically by using ANSYS (5.4) program.

To examine the validity of the derived equations for forced vibration response for composite laminated shells, a comparison study is done with a shallow spherical shell of [1]. By using the present analysis and finite element method in ANSYS (5.4), which shows good agreement between the results for the central deflection of two layer ( $0 / 90$ ) cross ply laminate shell, which are $(16.287(\mathrm{~mm})$ using GTT, $17.978(\mathrm{~mm})$ using ANSYS $)$, while it was $(\approx 16$ .537 (mm) taken from graph in [1]), its obvious that the difference between the published results
and the present work is $(1.533 \%)$, these results are shown in Fig. (1).

Maximum central displacements (W) for antisymmetric cross ply ( $0 / 90$ ) under different load-time functions are listed in Table (1), from which its obvious that maximum displacement of this cylindrical shell occurs when it is under the rectangular-pulse, the variations of these displacements as function of time are plotted in Fig.(2), as a result then stresses ( $\sigma 1$ ), ( $\sigma 2$ ), also have their maximum values under this pulse and their variation with time are shown in Figs.(3 and 4) respectively, in all figures the plotted stress component are taken as: $\sigma_{1}=\frac{\sigma_{1}\left(\frac{\mathrm{~L}}{2}\right), \frac{\pi}{2},\left(\frac{\mathrm{H}}{2}\right)}{\mathrm{q}_{0}}$ $\sigma_{2}=\frac{\sigma_{2}\left(\frac{\mathrm{~L}}{2}\right) ; \frac{\pi}{2},\left(\frac{\mathrm{H}}{2}\right)}{\mathrm{q}_{0}}$

The maximum central displacements for symmetric cross ply (0/90/0) laminated cylindrical shells are developed in Table (2), rectangularpulse, here also gives maximum central displacement and therefore maximum stress components to the cylindrical shells. The variation of central displacements under these load-time functions are shown in Fig. (5), while the variation of stress components are shown in Figs. (6 and 7). Further, the amplitudes are smaller for symmetric cross ply than that for antisymmetric cross ply laminates, ( 22.57 and 22.36 mm ) respectively.

Similar results are presented for central displacements (W) and normal stress ( $\sigma 1$ ), ( $\sigma 2$ ), $(\sigma 3)$ and transverse shear stress $(\sigma 4),(\sigma 5),(\sigma 6)$ for isotropic cylindrical shells in Table (3). Rectangular-pulse dynamic load also causes the
maximum central displacement for this shell (its variation with time under different load functions are shown in Fig.(8), but it is smaller than that for both types of cross-ply cylindrical shells. Therefore, the stress components are also smaller, the variation of the isotropic shell stress components with time are as shown in Figs. (9 and 10).

Geometrical dimensions for the worked cases are, for spherical shell: $(a=b=20, R 11=R 22=5 a$, $\mathrm{H}=2$ ), for laminated closed cylindrical shell: ( $\mathrm{R}=\mathrm{L}=20 \mathrm{H}=2$ ), while load amplitude $\mathrm{qo}=2000 \mathrm{MPa}$ for cylindrical shells and qo $=13.788 \mathrm{MPa}$ for spherical shell, time duration for all load-time functions $\mathrm{TD}=.003 \mathrm{sec}$.. Also in all calculations, material properties of the shells are listed in Table (4).

Table (1): Central displacement and stress components for two- layered (0/90) closed cylindrical shell and four types of pulses.

| Load-time <br> Function | W <br> $(\mathrm{mm})$ | $\left(\sigma_{1} / \mathrm{q}_{\mathrm{o}}\right)$ | $\left(\sigma_{2} / \mathrm{q}_{\mathrm{o}}\right)$ | $\left(\sigma_{3} / \mathrm{q}_{\mathrm{o}}\right)$ | $\left(\sigma_{4} / \mathrm{q}_{\mathrm{o}}\right)$ | $\left(\sigma_{5} / \mathrm{q}_{\mathrm{o}}\right)$ | $\left(\sigma_{6} / \mathrm{q}_{\mathrm{o}}\right)$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Rectangular-pulse | 22.57 | 45.12 | -175.62 | -17.91 | 1.22 | $9.33 \mathrm{e}-1$ | 13.32 |
| Triangular-pulse | 17.78 | 37.00 | -107.22 | -11.18 | 1.00 | $8.24 \mathrm{e}-1$ | 9.97 |
| Sine-pulse | 12.09 | 23.02 | -94.60 | -10.14 | $6.60 \mathrm{e}-1$ | $5.47 \mathrm{e}-1$ | 5.96 |
| Ramp-Constant | 12.11 | 23.05 | -94.83 | -10.17 | $6.61 \mathrm{e}-1$ | $5.48 \mathrm{e}-1$ | 5.97 |

Table (2): Central displacement and stress components for three- layered (0/90/0) closed cylindrical shell and four types of pulses.

| Load-time <br> Function | W <br> $(\mathrm{mm})$ | $\left(\sigma_{1} / \mathrm{q}_{\mathrm{o}}\right)$ | $\left(\sigma_{2} / \mathrm{q}_{\mathrm{o}}\right)$ | $\left(\sigma_{3} / \mathrm{q}_{\mathrm{o}}\right)$ | $\left(\sigma_{4} / \mathrm{q}_{\mathrm{o}}\right)$ | $\left(\sigma_{5} / \mathrm{q}_{\mathrm{o}}\right)$ | $\left(\sigma_{6} / \mathrm{q}_{\mathrm{o}}\right)$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Rectangular-pulse | 22.36 | 112.53 | -253.86 | -38.93 | $3.37 \mathrm{e}-1$ | 14.22 | 34.54 |
| Triangular-pulse | 20.14 | 104.36 | -201.58 | -30.21 | $2.58 \mathrm{e}-1$ | 12.88 | 32.05 |
| Sine-pulse | 11.45 | 52.45 | -120.91 | -20.16 | $1.76 \mathrm{e}-1$ | 7.39 | 14.66 |
| Ramp-Constant | 11.44 | 52.40 | -120.84 | -20.15 | $1.76 \mathrm{e}-1$ | 7.38 | 14.65 |

Table (3): Central displacement and stress components for isotropic (steel) closed cylindrical shell and four types of pulses.

| Load-time Function |  | $\begin{gathered} \mathrm{W} \\ (\mathrm{~mm}) \end{gathered}$ | $\left(\sigma_{1} / q_{0}\right)$ | ( $\sigma_{1} / \mathrm{q}_{\mathrm{o}}$ ) | ( $\sigma_{3} / \mathrm{q}_{\mathrm{o}}$ ) | $\left(\sigma_{4} / \mathrm{q}_{0}\right)$ | $\left(\sigma_{5} / \mathrm{q}_{0}\right)$ | $\left(\sigma_{6} / \mathrm{q}_{\mathrm{o}}\right)$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Rectangular-pulse |  | 4.16 | -34.05 | -99.15 | -61.52 | 5.38e- | 1.86 | 13.52 |
| Triangular-pulse |  | 3.79 | -29.48 | -85.786444 | -54.10 | $4.65 \mathrm{e}-$ | 1.61 | 11.43 |
| Sine-pulse |  | 2.09 | -17.42 | -49.89 | -31.15 | $2.75 \mathrm{e}-$ | $9.47 \mathrm{e}-1$ | 5.70 |
| Ramp-Constant |  | 2.10 | -17.51 | -50.14 | -31.31 | 2.76e- | $9.52 \mathrm{e}-1$ | 5.74 |
|  |  |  |  |  |  |  |  |  |
| Table (4): Material properties. |  |  |  |  |  |  |  |  |
| Material properties | Present work |  |  | Steel |  | [1] |  |  |
| $\mathrm{E}_{1}$ | 132.38 e 3 MPa |  |  | 20.6 e 4 MPa |  | 132.38 e 3 MPa |  |  |
| $\mathrm{E}_{2}=\mathrm{E}_{3}$ | 10.75 e 3 MPa |  |  | 20.6 e 4 MPa |  | 10.75 e 3 MPa |  |  |
| $\mathrm{G}_{12}=\mathrm{G}_{13}$ | 5.653 e 3 MPa |  |  | 8 C 4 MPa |  | 5.653 e 3 MPa |  |  |
| $\mathrm{G}_{23}$ | 3.608 e 3 MPa |  |  | 8e4MPa |  | 3.608 e 3 MPa |  |  |
| $v_{12}=v_{13}$ | . 24 |  |  | . 3 |  | . 24 |  |  |
| $v_{23}$ | . 49 |  |  | . 3 |  | . 49 |  |  |
| $\rho$ | $1.32288 \mathrm{e}-9\left(\mathrm{~N}-\mathrm{s}^{2} / \mathrm{mm}^{4}\right)$ |  |  | $7.85 \mathrm{e}-9\left(\mathrm{~N}-\mathrm{s}^{2} / \mathrm{mm}^{4}\right)$ |  | $1.32288 \mathrm{e}-9\left(\mathrm{~N}-\mathrm{s}^{2} / \mathrm{mm}^{4}\right)$ |  |  |

## 4. Conclusions:

The prominent points in this work are as follows:
1-A general third order shell theory (GTT) is developed to derive the governing equations for forced vibration of simply supported cylindrical shells, for first time, and these equations are solved using Navier's method.
2-Good agreement between the results obtained by using GTT in present work with those obtained by other researchers using FEM for analyzing the dynamic behavior of laminated spherical shells, maximum percentage of error is ( $1.533 \%$ ).
3-The response due to the rectangular loading has largest amplitude than that of other types of loading.
4-Symmetric cross-ply laminates have smaller amplitudes than that for antisymmetric one.

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## Nomenclature

| a, b | Dimensions of shell. |
| :---: | :---: |
| $\mathrm{A}_{\mathrm{mn}}, \mathrm{B}_{\mathrm{mn}}, \mathrm{C}_{\mathrm{mn}}, \mathrm{D}_{\mathrm{mn}}, \mathrm{E}_{\mathrm{mn}}$, Arbitrary constants$\mathrm{F}_{\mathrm{m}}$ |  |
|  |  |
| B | constant |
| $\mathrm{C}_{\mathrm{ij}}$ | stiffness matrix elements |
| $\mathrm{E}_{1}, \mathrm{E}_{2}, \mathrm{E}_{3}$ | Elastic Modulus components (GPa) |
| $\mathrm{f}_{\mathrm{mn}}(\mathrm{t})$ | Generalized force (N) |
| $\mathrm{g}_{1}, \mathrm{~g}_{2}$ | Body forces (N) |
| $\mathrm{G}_{12}, \mathrm{G}_{13}, \mathrm{G}_{23}$ | Shear modulus components (GPa) |
| H | Thickness (mm) |
| K | Kinetic energy |
| L | Cylinder length (mm) |
| m, n | indices |
| $\mathrm{N}_{\mathrm{i}}, \mathrm{M}_{\mathrm{i}}, \mathrm{P}_{\mathrm{i}}, \mathrm{S}_{\mathrm{i}}(\mathrm{i}=1,2,3,6)$ | Resultant reactions (N/mm),(N.mm) |
| $\mathrm{m}_{1}, \mathrm{~m}_{2}, \mathrm{~m}_{3}, \mathrm{~m}_{4}$ | Body moments (N.mm) |
| $\mathrm{Q}_{\mathrm{i}}, \mathrm{K}_{\mathrm{i}}(\mathrm{i}=4,5)$ | Resultant reactions ( $\mathrm{N} / \mathrm{mm}$ ) |
| $\mathrm{Q}_{\mathrm{ij}}$ | Elastic stiffness coefficients |
| q | Distributed transverse load ( $\mathrm{N} / \mathrm{mm}^{2}$ ) |
| R | Cylinder radius (mm) |
| $\mathrm{R}_{11}, \mathrm{R}_{22}$ | Principal radii of curvature of shell (mm) |
| U | Potential energy |
| $\theta_{1}, \theta_{2}, \theta_{3}$ |  |
| Z | Distance from neutral axis (mm) |
| $\varepsilon_{1,2,3,4,5,6}$ | Strain components in principal directions |
| $\mathrm{v}_{12}, \mathrm{v}_{13}, \mathrm{v}_{23}$ | Poison s ratios |
| $\rho$ | Density ( $\mathrm{Ns}^{2} / \mathrm{mm}^{4}$ ) |
| $\omega$ | Frequency ( $\mathrm{rad} / \mathrm{s}$ ) |
| $\sigma_{1,2,3,4,5,6}$ | Stress components (MPa) in principal direction |



Figure (1) Variation of center deflection as a function of time, for two- layered (0/90) shallow spherical shell under triangular pulse.


Figure (2) Variation of central deflection as a function of time, for two- layered (0/90) closed cylindrical shell and four types of pulses.


Figure (3) Variation of ( $\sigma 1$ ) as a function of time, for two- layered ( $0 / 90$ ) closed cylindrical shell and four types of pulses.


Figure (4) Variation of $\left(\sigma_{2}\right)$ as a function of time, for two- layered ( $0 / 90$ ) closed cylindrical shell and four types of pulses.


Figure (5) Variation of central deflection as a function of time, for three- layered (0/90/0) closed cylindrical shell and four types of dynamic load.


Figure (6) Variation of $(\sigma 1)$ as a function of time, for three- layered ( $0 / 90 / 0$ ) closed cylindrical shell and four types of dynamic load.

|  |  |
| :---: | :---: |
| (a): Rectangular pulse | (b): Triangular pulse |
|  |  |
| (c): Sine pulse | (d): (Ramp-Constant) |

Figure (7) Variation of $\left(\sigma_{2}\right)$ as a function of time, for three- layered ( $0 / 90 / 0$ ) closed cylindrical shell and four types of dynamic load.


Figure (8) Variation of central deflection as a function of time, for isotropic (Steel) closed cylindrical shell and four types of dynamic load.


Figure (9) Variation of ( $\sigma_{1}$ ) as a function of time, for isotropic (steel) closed cylindrical shell and four types of dynamic load.


Figure (10) Variation of $\left(\sigma_{2}\right)$ as a function of time, for isotropic (steel) close cylindrical shell and four types of dynamic load.

# حل تحليلي مقترح للاسطو انات الطباقية المتلقة باستذدام نظرية الرقائق العامة 

## الثالثة

د. ـد وداد ابراهيم العزاوي<br>قسم الهندسة الميكانيكية<br>جامعة بغداد

د. محسن جبر جويج
قسم الهنسة الميكانيكية
جامعة النهرين

الخلاصة:
تم تطوير حل مشكلة الاستجابة الحظية للرقائق الاسطو انية المغلقة الطباقية ذات الاسناد البسيط المعرضة لضغط موزع ع بصورة منتظمة على سطهها الخارجي. هذه الحلول تمت باستخدام نظرية الرقائق الثالثة العامة. النبضة المستطيلة، النبضة المنلثة، النبضة الجييبة ونبضة (النغير الخطي- ثابت) كدو ال لتغير الحمل مع اللزمن تم در استها وتحصبل المعادلات الاتز ان اللازمة لها. الازاحة المركزية والاجهادات الاساسية تم بحثها للـختلف الطبقات المتقاطعة.

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